# The Characteristics of Ground Motions Caused by Blasting in the Peikang Area

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#### ABSTRACT

In recent years, the problem of the effects of ground vibration generated by blasting on nearby structures has received a lot of attention. To minimize or eliminate damages, we need a reliable basis on which to plan and conduct blasting operations. For this reason, we performed seven experiments in the Peikang area. The observed peak ground motion values for different weights of explosive and source-receiver distances were used to obtain the following attenuation equations:

Acceleration (cm sec<sup>-2</sup>):  $A = 6.1476R^{-1.6263}W^{1.0375}$ , Velocity (mm sec<sup>-1</sup>):  $V = 3.5315R^{-1.2664}W^{0.7836}$ .

where W represents the weight of explosive in gm, and R is the source-receiver distance in meters.

Additionally, the ground acceleration time histories of the test were transferred to a frequency domain to study their contents. From the spectra, we realize that the dominant frequency band of the Peikang area is lower than 10 Hz. There is also a second energy concentration in the 15-30 Hz band. The particle motions indicated that the high frequency vibration (larger than 15 Hz) is a body wave which came from the blast source and the low frequency vibration (lower than 10 Hz) is mainly a surface wave contribution which was generated by the local geological structure.

#### **1. INTRODUCTION**

The relationship between blasting operations and building damage is often a subject of argument when blasting is done in the vicinity of buildings. Until recently, however, there has been a dearth of published information relating blasting conditions to reliably documented damage in the Taiwan area. Engineers must, however, be able to determine the maximum weight of explosive that can be detonated without damaging adjacent structures. In June 1985, an explosive survey in the Peikang area was performed by the Institute of Earth Sciences, Academia Sinica and the Chinese Petroleum Corporation. In our study, a dynamite source was used and recordings were obtained at different hypocentral distances. The seismograms were analyzed to show the characteristics of the ground motion over time and the frequency domain. Finally, the relationships between ground motions, distances, and amount of dynamite are inferred for the Peikang area. Hopefully, our results will benefit the engineering community.



Fig. 1. Seismic survey system in this study.

## 2. FIELD OPERATION

In June 1985, we conducted a test in the Peikang area in central Taiwan. The observation system (Fig. 1) was composed of four SA-3000 accelerometers and DR-100 digital recorders from the Sprengnether Corporation and two FBA-13 accelerometers and PDR-2 digital recorders from the Kinemetrics Corporation. Both SA-3000 and FBA-13 accelerometers have three components. The arrangements of the two horizontal components are, respectively, parallel (L) and transverse (T) to the survey line. Totally, there were 18 channels for digital recording. The sampling interval for each channel is 0.01 seconds. At the same time, an analog recorder of HP7418A was used to monitor the experiment.

There were seven shots conducted in the Peikang area. Every shot hole did not have an iron casing. Table 1 illustrates the pertinent information about shot holes, weight of the explosive and the distance between the middle shot hole and the farthest station for each blast. The positions of the receivers were fixed, but the shot points were shifted to the west or northwest direction along the survey line.

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Shot Nc.	Arrangement <sup>1</sup> & (m) of shot	depth Cha hole	rge weight (kg)	Distance <sup>2</sup> (m)	Remark
1	N 0 10 Î 0 10 0 10		2 x 3	150	
2	№ 0 0 0 ↑ 10 10 10		2 x 3	250	
,3	N 0 0 0 Î 10 10 10		3 x 3	300	
·4	N O O O † 10 10 10		2 x 3	150	
5	N 0 0 0 ↑ 10 10 10		3 x 3	350	
6	N O O O † 10 10 10		1 x 3	150	shot hole shift 8 m to the north
7	N 0 0 0 † 5 5 5		1 x 3	150	shot hole shift 16m to the north

Table 1. Field surveys in the Peikang area.

Note: 1. Shot hole interval are fixed in 8 meters.

2. Distance between the farthest station and middle shot hole.

## 3. THE GROUND MOTIONS

Since field data were recorded on cassette tapes, they had to be played back and transferred to a 9-track tape first. The data were then edited to delete the occasional glitches. The baseline correction was also made at this stage. Figs. 2 and 3 plot some examples of the corrected acceleration time traces for two different charges. From these figures, we can see without difficulty that the ground motion attenuates with distance which occurs at a higher rate in the near field. Comparing the time histories of different charges (Figs. 2 and 3), we can also see that the ground motion is proportional to the weight of the explosive.

The waveforms had large low frequency ground rolling following the high frequency motions. The reason for this may be occurred by the underground geological structure and the properties of the strata in the survey area.

The peak ground acceleration and velocity are commonly used to represent the intensity of the ground motion for engineering design criteria. In this study, the acceleration was integrated in the frequency domain to compute the ground velocity records. From the recorded time histories, the largest peak acceleration or velocity of the three components was chosen for every site and tabulated in



Fig. 2. Vertical acceleration waveforms for the first blast with 6 kg of dynamite in the Peikang area.



Fig. 3. Vertical acceleration waveforms for the sixth blast with 3 kg of dynamite in the Peikang area.

Station		Cm1	cm2	Cm 3	am 4	0.005	cme	W
Shot No.		511	STZ	STJ	514	515	510	(kg)
1	Acc.	470.55 <sup>v</sup>	90.35°	58.28 <sup>v</sup>	32.63 <sup>v</sup>	23.09 <sup>v</sup>		
	Vel.	70.57 <sup>L</sup>	17.48 <sup>v</sup>	15.90 <sup>v</sup>	11.46 <sup>v</sup>	8.21 <sup>v</sup>	·	6
	Dis.	26.93	50.99	75.66	100.50	125.40		
2	Acc.	21.80 <sup>v</sup>	0 v 1		11.56 <sup>v</sup>	12.13 <sup>v</sup>	7.78 <sup>V</sup>	
	Vel.	9.27¥		,	3.874	3.66 <sup>1</sup>	3.94	6
	Dis.	117.43			192.26	217.23	242.21	
	Acc.		·	14.97 <sup>v</sup>	14.09 <sup>v</sup>	20.40 <sup>v</sup>		
3	Vel.		·	4.53 <sup>v</sup>	4.02 <sup>v</sup>	. 3.99 <sup>v</sup>		. 9
	Dis.			217.23	242.21	267.19		
4	Acc.	151.85 <sup>L</sup>	77.33♥		28.66 <sup>v</sup>	18.12 <sup>v</sup>	6.74 <sup>v</sup>	
	Vel.	56.89 <sup>L</sup>	14.10 <sup>v</sup>		<b>11.8</b> 9 <sup>v</sup>	8.72 <sup>v</sup>	3.75 <sup>v</sup>	6
	Dis.	19.72	43.17		92.54	117.43	142.35	
	Acc.			13.19 <sup>v</sup>	14.78 <sup>v</sup>	18.16 <sup>v</sup>	3.17 <sup>v</sup>	
5	Vel.	<b>.</b>		3.09 <sup>L</sup>	3.53 <sup>v</sup>	3.38 <sup>v</sup>	1.74 <sup>v</sup>	9
	Dis.			267.19	292.17	317.16	342.15	
	Acc.	451.93 <sup>v</sup>	69.13 <sup>v</sup>	41.90 <sup>v</sup>	20.32 <sup>v</sup>	16.30 <sup>v</sup>		
6	Vel.	55.49 <sup>v</sup>	9.27 <sup>v</sup>	9.04 <sup>v</sup>	6.02 <sup>v</sup>	4.70 <sup>v</sup>		3
	Dis.	21.28	43.91	68.21	92.89	117.70		
7	Acc.	222.44 <sup>v</sup>	32.25 <sup>v</sup>	21.38 <sup>v</sup>	17.67*		4.41 <sup>v</sup>	
	Vel.	38.34 <sup>v</sup>	10.09 <sup>v</sup>	9.51 <sup>v</sup>	7.00*		2.32*	3
	Dis.	23.87	45.22	69.07	93.51		142.99	

Tabl	le	2	. Pea	k	ground	motion	of	each	site	in	the	Peikang	area.
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Note: 1. The unit of Dis. is in meters, Acc. is in gal, and Vel. is in mm/sec.

2. V and L represent the peak occurred component.

Table 2. Most of the largest peak amplitudes in the Peikang area appear in the vertical (V) component.

# 4. FREQUENCY CONTENTS



Fig. 4. Normalized Fourier spectra of the V component for the first blast in the Peikang area.

In this study of the vibration problem, frequency contents are a very important characteristic. Fourier spectra are very useful in determining the predominant frequencies, and in general, for studying the distribution of vibrational energy over the entire frequency range. A fast Fourier transformation (Cooley and Tukey, 1965) was used for calculating the amplitude spectra. A typical spectra is shown in Fig. 4, for the first blast of the survey. Each spectrum was normalized to its peak value. The dominant frequency band is lower than 10 Hz. However, there was also a second energy concentration in the 15-30 Hzband,

For the sake of determining the characteristics of these two frequency bands, we used a band pass filter to split it. Fig. 5 is the time history lower than 10 Hz, for the first blast. Fig. 6 is the time history larger than 15 Hz, for the first blast. Comparing these with Fig. 2, we find that the low frequency



Fig. 5. Time histories of the low frequency (< 10 Hz) band of stations 1 and 2 for the first blast in the Peikang area.

vibration occurred after the high frequency vibration. At the same time, we plotted out the particle motions of these two records on Figs. 7 and 8. The higher frequency (larger than 15 Hz) band's particle motion was dominated by a vertical component (see Figs. 7b and 8b); the lower frequency (lower than 10 Hz) band's particle motion (see Figs. 7a and 8a) describes an ellipse, the motion being retrograde. Based on these particle motions, we can see that the high frequency vibrations are the body wave's motion which came from the source, and the low frequency vibration are the surface wave's motion which



Fig. 6. Time histories of the high frequency (> 15 Hz) band of stations 1 and 2 for the first blast in the Peikang area.

was generated by the local geological structure.

## 5. ATTENUATION EQUATIONS

The determination of the maximum safe explosive amount requires the knowledge of : 1. The attenuation equation of the ground motions resulting from blasting, whereby the intensity of ground vibration can be predicted on the basis of the weight of explosive, distance from the detonation point, and

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Fig. 7. (a) Particle motion of the low frequency  $(< 10 \ Hz)$  band of station 1 for the first blast in the Peikang area.



Fig. 7. (b) Particle motion of the high frequency (> 15 Hz) band of station 1 for the first blast in the Peikang area.





Fig. 8. (a) Particle motion of the low frequency (< 10 Hz) band of station 2 for the first blast in the Peikang area.





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dynamic properties of soil and rock, etc. 2. Reliable damage criteria for different types of structures, whereby the damage of a structure can be correlated with the intensity of the ground vibration.

Since 1935, many investigations have been conducted to study the effects of ground vibrations on structures and to formulate the attenuation equation of ground vibrations (Duvall and Fogelson, 1962; Northwood *et al.*, 1963; Naik, 1979; and Wiss, 1981). There is no universally accepted theoretical approach to derive the attenuation equation. Empirical approaches are presently used. The most widely accepted form is :

$$Y = kR^m W^n, \tag{1}$$

where Y is the peak amplitude of the vibration at a distance R from the detonation of an explosive charge of weight W. The constant k, m and n are associated with the properties of the transmitting medium and other variables (Dowding, 1977; Medearis, 1977; Chae, 1978; Shigekazu Uchiyama *et al.*, 1979).

If the attenuation equation of formula (1) is linear on a natural logirithm scale, then we can change it to :

$$lnY = a + blnR + clnW,$$
(2)

where a = lnk, b = m, and c = n. These constants a, b, and c can be calculated by the multiple linear regression method (Ang and Tang, 1975). The results are as follows,

$$lnA = 1.6763 - 1.5489 lnR + 1.0272 lnW \pm 0.5034,$$
(3)

$$lnV = 1.2225 - 1.2664 lnR + 0.7836 lnW \pm 0.2757, \tag{4}$$

where, A is the peak ground acceleration in  $cm \ sec^{-2}$ , V is the peak ground velocity in  $mm \ sec^{-1}$ , R is the source-receiver distance in meters, and W is the weight of explosive in gm.

If the uncertainties in the data are small, a fit to equation (2) will yield a good fit to equation (1). The problem comes when trying to optimize the fit for reasonable uncertainties. In effect, the reparameterization of equation (2)overemphasizes the uncertainties for large values of Y and tends to optimize the fit for the small values of Y. To compensate for this trend, we used trial-anderror to modify equations (3) and (4). The constants of equations (3) and (4)were used as initial values, and the variance value for testing the goodness of fit should be determined from the unmodified equation (1). Then, we obtained the following attenuation equations,

Acceleration 
$$(cm \ sec^{-2}) A = 6.1476 R^{-1.6263} W^{1.0375},$$
 (5)  
Velocity  $(mm \ sec^{-1})$ :  $V = 3.5315 R^{-1.2664} W^{0.7836},$  (6)







Fig. 10. Velocity attenuation curves in the Peikang area.

The corresponding attenuation curves are shown in Figs. 9 and 10.

#### 6. DISCUSSION AND CONCLUSIONS

Based on the time and frequency domain analysis of the ground motion records for the seven blasting experiments performed in the Peikang area, the principal results of this study are as follows:

1. From the time histories, we can see without difficultly that the amplitude of the ground motion is attenuated with distance, and is attenuated at a higher rate in the near field. In the meantime, there were large, low frequency ground motions after the high frequency wave's motion. From Table 2, we can find that most peaks occurred on the vertical component.

2. Based on the multiple linear regression of the observed peak ground motion values with different weights of explosive and hypocentral distances, we found that the attenuation equations for the Peikang survey area are:

Acceleration  $(cm \ sec^{-2}):A = 6.1476R^{-1.6263}W^{1.0375},$ 

Velocity  $(mm \ sec^{-1})$ :  $V = 3.5315 R^{-1.2664} W^{0.7836}$ ,

where W represents the weight of explosive in gm, and R is the hypocentral distance in meters.

3. From the Fourier spectra, we know that the vibrational energy of the Peikang area is concentrated mainly on a frequency band lower than 10 Hz. Except for this low frequency energy band, there was another energy band higher than 15 Hz.

4. After comparison of the particle motions of these two frequency bands, we can see that the high frequency vibration, which is larger than 15 Hz, is the body wave's motion that came from the blast source, and the low frequency vibration, which is lower than 10 Hz, is mainly the surface wave's motion that was generated by the local geological structure.

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# 北港地區爆炸引起的地表振動特性

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# 摘 要

近年來,一般民衆非常注意在結構物附近進行炸測所引起之地表振動問題,要減少或除 去災害,須建立一可行的準則,作為預先規劃和現場實施爆炸作業時之參考。1985年6月 我們在北港地區實施七次炸測試驗,由所收集人工爆炸引起的地表振動記錄,觀察不同炸藥 量,不同距離的最大振動值,並推導最大地表振動的理論衰減公式,其結果為:

加速度( $cm/sec^2$ ): A = 6.1476 R<sup>-1.6263</sup> W<sup>1.0375</sup> 速度(mm/sec): V = 3.5315 R<sup>-1.2644</sup> W<sup>0.7836</sup>

其中,W代表炸藥量,單位為克;R代表震源距,單位為公尺。

此外,為瞭解炸測產生之振動頻率內涵,我們也將炸測試驗所記錄之地動加速度歷時, 經富氏轉換求出其富氏振幅譜。由此可知北港地區之振動能量集中於10赫以下和大於15赫之 頻帶。此高頻能帶爲爆炸震源產生,而低頻能帶則爲局部地質之效應。另外,由兩區質點振 動之比較可知,15赫以上爲體波之振動,10赫以下爲表面波之振動。